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DYNAMIC ANALYSIS OF THE VEHICLE-STRUCTURE INTERACTION: A DIRECT AND EFFICIENT COMPUTER IMPLEMENTATION

Alvaro Azevedo¹*, Sérgio Neves² and Rui Calçada²

¹: Department of Civil Engineering
Faculty of Engineering
University of Porto
Rua Dr. Roberto Frias, s/n, 4200-465 Porto, PORTUGAL
e-mail: alvaro@fe.up.pt, web: http://www.fe.up.pt/~alvaro

²: Department of Civil Engineering
Faculty of Engineering
University of Porto
Rua Dr. Roberto Frias, s/n, 4200-465 Porto, PORTUGAL
e-mail: {sgneves,ruiabc}@fe.up.pt, web: http://www.fe.up.pt

Key words: Vehicle-Structure Interaction, Dynamic Analysis, Time History Analysis, Hilber-Hughes-Taylor Method, Alpha-Method, Finite Element Method.

Summary. The simulation of the dynamic behavior of a structure subjected to sets of moving loads originated by vehicles whose structural behavior is also considered corresponds to a task not efficiently addressed by standard finite element packages. The capability of solving this type of problems has been introduced in the FEMIX 4.0 computational code by means of an integrated formulation, which includes equilibrium and compatibility equations, with unknowns that consist on displacements and interaction forces. Each system of linear equations is efficiently solved by considering the characteristics of each submatrix of the coefficient matrix.
1. INTRODUCTION

The finite element simulation of the dynamic effects of moving loads on structures such as bridges can be performed with or without the consideration of the vehicle's own structure. When this is not taken into consideration only a set of moving loads has to be included in the structural model of the bridge. The simulation of the vehicle-structure requires the consideration of several independent meshes and their compatibilization in contact points. This compatibilization may require a connection between two nodal points, between a nodal point and a surface point or between two surface points. The first situation is simply a master/slave relationship between two degrees of freedom of the finite element mesh. The second situation requires the compatibilization of a nodal degree of freedom with the displacements of a point that is located in the surface of the finite element. These techniques have been implemented in FEMIX 4.0, which is a general purpose finite element computer program [1]. The third situation is not treated here.

The dynamic analysis of a structure can be performed by direct integration of the dynamic equilibrium equations by means of one of the classical time history methods (e.g., Newmark method, Wilson-θ method). A slight improvement of the Newmark method was proposed in [2]. This new algorithm is termed Hilber-Hughes-Taylor (HHT) method or alpha-method, and is adopted in the present work.

This paper describes the formulation of the contact between nodal points of the vehicle and internal points of a finite element. In each time step a linear behavior is assumed. Dynamic equilibrium equations in non prescribed degrees of freedom, in contact degrees of freedom and in prescribed degrees of freedom are separately developed. Contact compatibility equations between points of the vehicle and internal points of a finite element are also separately developed. All these equations constitute a single system of linear equations involving displacements, contact forces and reactions as unknowns. After the solution of this system of linear equations the displacements, velocities and accelerations at the current time step can be calculated and a new time step is started. This heterogeneous system of linear equations can be efficiently solved by means of the consideration of several submatrices with specific characteristics.

A numerical application is presented to validate the formulation described in this paper.

2. FEMIX COMPUTER CODE

The HHT method has been implemented in the FEMIX 4.0 computer code [1], whose purpose is the analysis of structures by the Finite Element Method (FEM). This code is based on the displacement method, being a large library of types of finite elements already available, namely 3D frames and trusses, plane stress elements, flat or curved elements for shells, and 3D solid elements. Linear elements may have two or three nodes, plane stress and shell elements may be 4, 8 or 9-noded and 8 or 20 noded hexahedra may be used in 3D solid analyses. This element library is complemented with a set of point, line and surface springs.
that model elastic contact with the supports, and also a few types of interface elements to model inter-element contact. Embedded line elements can be added to other types of elements in order to model reinforcement bars. All these types of elements can be simultaneously included in the same analysis, with the exception of some incompatible combinations. The analysis may be static or dynamic and the material behavior may be linear or nonlinear. Data input is facilitated by the possibility of importing CAD models. Post processing is performed with a general purpose scientific visualization program named drawmesh [1].

Advanced numerical techniques are available, such as the Newton-Raphson method combined with arc-length techniques and path dependent or independent algorithms. When the size of the systems of linear equations is very large, a preconditioned conjugate gradient method can be advantageously used.

In the context of the dynamic analysis of structures with moving loads and vehicle-structure interaction the behavior of the materials is considered linear and the displacements are assumed to be small enough to avoid geometrically nonlinear phenomena.

The following section provides a detailed description of the formulation of the HHT method in the context of a dynamic analysis with vehicle-structure interaction.

3. HHT METHOD WITH VEHICLE-STRUCTURE INTERACTION

A simple example is used to introduce the types of degrees of freedom that are considered in the formulation of the vehicle-structure interaction in the context of a time step of the HHT method (see Figure 1). On the right, a simply supported beam with two spans \( B_1 \) and \( B_2 \) is subjected to the contact of a vehicle, shown on the left. The structure of the vehicle is also composed of two beams \( B_3 \) and \( B_4 \). Nodes 7, 8 and 9 are internal points of the beam \( B_1 \). The location of these nodes may change between time steps, depending on the position of the vehicle. Eventual gaps between both structures \( g_i \) can be easily considered in the compatibility equations, as will be shown later.

![Figure 1. Vehicle and structure: beams and nodal points.](image)

In each nodal point two degrees of freedom are considered (vertical displacement and rotation). Figure 2 shows the generalized displacements in nodal points (1 to 12), the generalized displacements of the contact points of the structure (13, 14 and 15), the interaction forces in the vehicle \( (X_7, X_9 \text{ and } X_{11}) \) and the interaction forces in the structure \( (Y_{13}, Y_{14} \text{ and } Y_{15}) \). The interaction only involves the translational degrees of freedom.
The following classification of the degrees of freedom is considered:

- $F$ – free;
- $X$ – interaction (vehicle);
- $P$ – prescribed;
- $Y$ – interaction (structure).

This classification is used later in this section.

In the context of the HHT method, the dynamic equilibrium equation that involves the degrees of freedom in nodal points (1 to 12) is the following

$$ M \ddot{u}^c + (1+\alpha) C \ddot{u}^c - \alpha C \dot{u}^p + (1+\alpha) K \dot{u}^c - \alpha K \dot{u}^p = (1+\alpha) F^c - \alpha F^p $$

In this equation $M$ is the mass matrix, $C$ is the damping matrix, $K$ is the stiffness matrix, $F$ are the applied generalized forces, $\dot{u}$ are the generalized displacements and $\alpha$ is the main parameter of the HHT method. When $\alpha=0$ the HHT method reduces to the Newmark method, and for other values of the parameter $\alpha$ numerical energy dissipation is introduced in the higher modes. The superscript $c$ indicates the current time step ($t+\Delta t$) and the superscript $p$ indicates the previous one ($t$).

According to Figure 2 and to the classification indicated above, the $F$ type degrees of freedom are the following: 2, 4, 6, 8, 10 and 12. The $X$ type degrees of freedom correspond to the "supports" of the separated vehicle-structure, being the following: 7, 9 and 11. The $P$ type degrees of freedom are the main structural supports 1, 3 and 5. The $Y$ type degrees of freedom 13, 14 and 15 consist on the internal displacements of beam $B1$ at the contact points.

According to this classification of degrees of freedom, Eq. (1) can be expanded by considering several submatrices, yielding
In the $F$ type degrees of freedom,
\[ F_F = P_F + d_{FY} Y_Y \]  
(3)

being $P$ the external loads applied in correspondence with each degree of freedom. Each component $d_{ij}$ of $d_{FY}$ corresponds to the nodal force in the $F$ type degree of freedom $i$, which is equivalent to a single load consisting of a unitary value of $Y_j$ (see Figure 2).

In the $X$ type degrees of freedom,
\[ F_X = P_X + L_{XX} X_X \]  
(4)

being $L_{XX}$ the identity matrix with an appropriate size.

In the $P$ type degrees of freedom,
\[ F_P = P_P + d_{PY} Y_Y + R_P \]  
(5)

being $R_P$ the reactions.

According to Figure 2, equilibrium equations in the contact degrees of freedom can be written, yielding
\[ Y_Y = -X_X \]  
(6)

Since the number of $Y$ type degrees of freedom coincides with the number of $X$ type degrees of freedom, the subscript $Y$ may be replaced with $X$.

Eq. (6) is then substituted into Eq. (2), leading to
Eq. (7) is equivalent to the following three equations

\[
\begin{pmatrix}
M_{FF} & M_{FX} & M_{FP} \\
M_{XF} & M_{XX} & M_{XP} \\
M_{PF} & M_{PX} & M_{PP}
\end{pmatrix}
\begin{pmatrix}
\ddot{u}_F \\
\ddot{u}_X \\
\ddot{u}_P
\end{pmatrix}
\]

\[\quad + (1 + \alpha)
\begin{pmatrix}
C_{FF} & C_{FX} & C_{FP} \\
C_{XF} & C_{XX} & C_{XP} \\
C_{PF} & C_{PX} & C_{PP}
\end{pmatrix}
\begin{pmatrix}
\dot{u}_F \\
\dot{u}_X \\
\dot{u}_P
\end{pmatrix}
\]

\[\quad + (1 + \alpha)
\begin{pmatrix}
K_{FF} & K_{FX} & K_{FP} \\
K_{XF} & K_{XX} & K_{XP} \\
K_{PF} & K_{PX} & K_{PP}
\end{pmatrix}
\begin{pmatrix}
\ddot{u}_F \\
\ddot{u}_X \\
\ddot{u}_P
\end{pmatrix}
\]

\[= (1 + \alpha)
\begin{pmatrix}
P^c_F \\
P^c_X \\
P^c_P
\end{pmatrix}
\]

\[\quad - \alpha
\begin{pmatrix}
P^p_F \\
P^p_X \\
P^p_P
\end{pmatrix}
\]

\[\quad + (1 + \alpha)
\begin{pmatrix}
-d_{FX} & 0 \\
0 & -d_{XX} & X^c_X \\
0 & -d_{PX} & 0
\end{pmatrix}
\begin{pmatrix}
0 \\
0 \\
R^c_P
\end{pmatrix}
\]

(1 + \alpha) \begin{pmatrix} P^c_F \\ P^c_X \\ P^c_P \end{pmatrix} - \alpha \begin{pmatrix} P^p_F \\ P^p_X \\ P^p_P \end{pmatrix} + (1 + \alpha) \begin{pmatrix} -d_{FX} \\ 0 \\ -d_{PX} \end{pmatrix} \begin{pmatrix} 0 \\ 0 \\ R^c_P \end{pmatrix} = (1 + \alpha) \begin{pmatrix} \ddot{X}^c_X + \alpha d_{FX} \ddot{X}^p_X \end{pmatrix}

(8)

\[
\begin{pmatrix}
M_{FF} & M_{FX} & M_{FP} \\
M_{XF} & M_{XX} & M_{XP} \\
M_{PF} & M_{PX} & M_{PP}
\end{pmatrix}
\begin{pmatrix}
\ddot{u}_F \\
\ddot{u}_X \\
\ddot{u}_P
\end{pmatrix}
\]

\[\quad + (1 + \alpha)
\begin{pmatrix}
C_{FF} & C_{FX} & C_{FP} \\
C_{XF} & C_{XX} & C_{XP} \\
C_{PF} & C_{PX} & C_{PP}
\end{pmatrix}
\begin{pmatrix}
\dot{u}_F \\
\dot{u}_X \\
\dot{u}_P
\end{pmatrix}
\]

\[\quad + (1 + \alpha)
\begin{pmatrix}
K_{FF} & K_{FX} & K_{FP} \\
K_{XF} & K_{XX} & K_{XP} \\
K_{PF} & K_{PX} & K_{PP}
\end{pmatrix}
\begin{pmatrix}
\ddot{u}_F \\
\ddot{u}_X \\
\ddot{u}_P
\end{pmatrix}
\]

\[= (1 + \alpha)
\begin{pmatrix}
P^c_F \\
P^c_X \\
P^c_P
\end{pmatrix}
\]

\[\quad - \alpha \begin{pmatrix}
P^p_F \\
P^p_X \\
P^p_P
\end{pmatrix}
\]

\[\quad + (1 + \alpha) \begin{pmatrix} -d_{FX} \\ 0 \\ 0 \end{pmatrix} \begin{pmatrix} 0 \\ 0 \\ R^c_P \end{pmatrix}
\]

(9)

\[
\begin{pmatrix}
M_{FF} & M_{FX} & M_{FP} \\
M_{XF} & M_{XX} & M_{XP} \\
M_{PF} & M_{PX} & M_{PP}
\end{pmatrix}
\begin{pmatrix}
\ddot{u}_F \\
\ddot{u}_X \\
\ddot{u}_P
\end{pmatrix}
\]

\[\quad + (1 + \alpha)
\begin{pmatrix}
C_{FF} & C_{FX} & C_{FP} \\
C_{XF} & C_{XX} & C_{XP} \\
C_{PF} & C_{PX} & C_{PP}
\end{pmatrix}
\begin{pmatrix}
\dot{u}_F \\
\dot{u}_X \\
\dot{u}_P
\end{pmatrix}
\]

\[\quad + (1 + \alpha)
\begin{pmatrix}
K_{FF} & K_{FX} & K_{FP} \\
K_{XF} & K_{XX} & K_{XP} \\
K_{PF} & K_{PX} & K_{PP}
\end{pmatrix}
\begin{pmatrix}
\ddot{u}_F \\
\ddot{u}_X \\
\ddot{u}_P
\end{pmatrix}
\]

\[= (1 + \alpha)
\begin{pmatrix}
P^c_F \\
P^c_X \\
P^c_P
\end{pmatrix}
\]

\[\quad - \alpha \begin{pmatrix}
P^p_F \\
P^p_X \\
P^p_P
\end{pmatrix}
\]

\[\quad + (1 + \alpha) \begin{pmatrix} -d_{FX} \\ 0 \\ 0 \end{pmatrix} \begin{pmatrix} 0 \\ 0 \\ R^c_P \end{pmatrix}
\]
By placing all terms containing unknown variables in the first member, Eqs (8) and (9) result in

\[
\begin{align*}
M_{PF} \ddot{u}_F^c + M_{FX} \ddot{u}_X^c + M_{PP} \ddot{u}_P^c + (1 + \alpha) C_{PF} \dot{u}_F^c + (1 + \alpha) C_{FX} \dot{u}_X^c + (1 + \alpha) C_{PP} \dot{u}_P^c \\
- \alpha C_{PF} \ddot{u}_F^p - \alpha C_{FX} \ddot{u}_X^p - \alpha C_{PP} \ddot{u}_P^p + (1 + \alpha) K_{PF} u_F^c + (1 + \alpha) K_{FX} u_X^c + (1 + \alpha) K_{PP} u_P^c \\
- \alpha K_{PF} u_F^p - \alpha K_{FX} u_X^p - \alpha K_{PP} u_P^p = \\
(1 + \alpha) P_F^c - \alpha P_F^p - (1 + \alpha) d_{FX} X^c_F + a d_{FX} X^p_F + (1 + \alpha) R_P^c - \alpha R_P^p
\end{align*}
\]

which is equivalent to Eq. (10).
Eq. (11) can be written as

\begin{equation}
\mathbf{M}_{FF} \dddot{\mathbf{u}}^c + \mathbf{M}_{FX} \dddot{\mathbf{u}}^c + (1+\alpha) \mathbf{C}_{FF} \dot{\mathbf{u}}^c + (1+\alpha) \mathbf{C}_{FX} \dot{\mathbf{u}}^c + (1+\alpha) \mathbf{d}_{FX} \mathbf{X}^c = \mathbf{F}_F
\end{equation}

being \( \mathbf{F}_F \) defined by

\begin{equation}
\mathbf{F}_F = (1+\alpha) \mathbf{P}_F - \alpha \mathbf{P}_F^p + \alpha \mathbf{d}_{FX} \mathbf{X}_F - \mathbf{M}_{FF} \dddot{\mathbf{u}}^c
\end{equation}

According to the Newmark method, the velocity and the displacement in the \( F \) type degrees of freedom, at the current time step \( t + \Delta t \), can be defined as [3]

\begin{equation}
\dot{u}_F^c = \dot{u}_F^p + \left[ (1-\gamma) \ddot{u}_F^p + \gamma \dddot{u}_F^c \right] \Delta t
\end{equation}

\begin{equation}
\mathbf{u}_F^c = \mathbf{u}_F^p + \ddot{\mathbf{u}}_F^c \Delta t + \left[ \frac{1}{2} \mathbf{\beta} \left( \dddot{\mathbf{u}}_F^p + \mathbf{\beta} \dddot{\mathbf{u}}_F^c \right) \right] \Delta t^2
\end{equation}

These equations are also used in the HHT method. The parameter \( \gamma \) defines a linear weighting between the influence of the initial and final accelerations on the velocity variation and the parameter \( \beta \) defines a similar weighting of the accelerations on the displacement variation. These parameters influence the stability and accuracy of the HHT method.

Solving Eq. (17) for \( \dddot{u}_F^c \) gives

\begin{equation}
\dddot{u}_F^c = \frac{1}{\mathbf{\beta} \Delta t} \frac{1}{\mathbf{\beta} \Delta t^2} \dddot{u}_F^c - \frac{1}{\mathbf{\beta} \Delta t} \dddot{u}_F^c - \frac{1}{\mathbf{\beta} \Delta t} \dddot{u}_F^p - \frac{1}{\mathbf{\beta} \Delta t} \dddot{u}_F^c
\end{equation}

Substituting \( \dddot{u}_F^c \) given by Eq. (18) into Eq. (16) yields

\begin{equation}
\dot{u}_F^c = \dot{u}_F^p + (1-\gamma) \ddot{u}_F^p \Delta t + \gamma \left[ \frac{1}{\mathbf{\beta} \Delta t} \dddot{u}_F^c - \frac{1}{\mathbf{\beta} \Delta t} \dddot{u}_F^p - \frac{1}{\mathbf{\beta} \Delta t} \dddot{u}_F^c - \frac{1}{\mathbf{\beta} \Delta t} \dddot{u}_F^p - \frac{1}{\mathbf{\beta} \Delta t} \dddot{u}_F^c \right] \Delta t
\end{equation}

This equation can be rewritten as

\begin{equation}
\dot{u}_F^c = \frac{\gamma}{\mathbf{\beta} \Delta t} \dddot{u}_F^c - \frac{\gamma}{\mathbf{\beta} \Delta t} \dddot{u}_F^p + \dddot{u}_F^p \left( 1 - \frac{\gamma}{\mathbf{\beta}} \right) \Delta t + \dddot{u}_F^c \left[ 1 - \gamma - \frac{\gamma}{2} + \gamma \right]
\end{equation}

which is equivalent to
\[ u_F^c = \frac{\gamma}{\beta \Delta t} u_F^c - \frac{\gamma}{\beta \Delta t^2} u_F^p + \frac{1}{\beta \Delta t} \frac{1}{2} - \frac{1}{2 \beta} \] (21)

By replacing \( F \) with \( X \) in Eq.s (16) and (17), and performing a similar rearrangement, one has, by analogy with Eq.s (18) and (21),

\[ u_X^c = \frac{1}{\beta \Delta t} u_X^c - \frac{1}{\beta \Delta t^2} u_X^p + \frac{1}{\beta \Delta t} \frac{1}{2} - \frac{1}{2 \beta} \] (22)

\[ u_X^p = \frac{\gamma}{\beta \Delta t} u_X^c - \frac{\gamma}{\beta \Delta t^2} u_X^p + \left( 1 - \frac{\gamma}{\beta} \right) u_X^p + \Delta t \left( 1 - \frac{\gamma}{2 \beta} \right) \] (23)

The substitution of Eq.s (18), (21), (22) and (23) into Eq. (14) yields

\[ \begin{align*}
M_{FF} & \left[ \frac{1}{\beta \Delta t^2} u_F^c - \frac{1}{\beta \Delta t^2} u_F^p - \frac{1}{\beta \Delta t} \frac{1}{2} - \frac{1}{2 \beta} \right] \\
+ M_{FX} & \left[ \frac{1}{\beta \Delta t^2} u_X^c - \frac{1}{\beta \Delta t^2} u_X^p - \frac{1}{\beta \Delta t} \frac{1}{2} - \frac{1}{2 \beta} \right] \\
+ (1 + \alpha) C_{FF} & \left[ \frac{\gamma}{\beta \Delta t} u_F^c - \frac{\gamma}{\beta \Delta t} u_F^p + \left( 1 - \frac{\gamma}{\beta} \right) u_F^p + \Delta t \left( 1 - \frac{\gamma}{2 \beta} \right) u_F^p \right] \\
+ (1 + \alpha) C_{FX} & \left[ \frac{\gamma}{\beta \Delta t} u_X^c - \frac{\gamma}{\beta \Delta t} u_X^p + \left( 1 - \frac{\gamma}{\beta} \right) u_X^p + \Delta t \left( 1 - \frac{\gamma}{2 \beta} \right) u_X^p \right] \\
+ (1 + \alpha) K_{FF} u_F^c + (1 + \alpha) K_{FX} u_X^c + (1 + \alpha) d_{FX} X_X^c = F_F
\] (24)

Rearranging this equation one has

...
which is equivalent to the following compact form

$$\overline{K}_F u_F^c + \overline{K}_X u_X^c + (1 + \alpha) d_{FX} X_X^c = \overline{F}_F$$  \hspace{2cm} (26)$$

being

$$\overline{K}_F = A_0 M_{FF} + (1 + \alpha) A_1 C_{FF} + (1 + \alpha) K_{FF}$$  \hspace{2cm} (27)$$

$$\overline{K}_X = A_0 M_{FX} + (1 + \alpha) A_1 C_{FX} + (1 + \alpha) K_{FX}$$

$$\overline{F}_F = \overline{F}_F + M_{FF} \left[ A_0 u_F^p + A_2 \dot{u}_F^p + A_1 \ddot{u}_F^p \right] + M_{FX} \left[ A_0 u_X^p + A_2 \dot{u}_X^p + A_1 \ddot{u}_X^p \right] + (1 + \alpha) C_{FF} \left[ A_0 u_F^p + A_2 \dot{u}_F^p + A_1 \ddot{u}_F^p \right] + (1 + \alpha) C_{FX} \left[ A_0 u_X^p + A_2 \dot{u}_X^p + A_1 \ddot{u}_X^p \right]$$  \hspace{2cm} (28)$$

$$A_0 = \frac{1}{\beta \Delta t^2} \hspace{1cm} A_1 = \frac{\gamma}{\beta \Delta t} \hspace{1cm} A_2 = \frac{1}{\beta \Delta t}$$

$$A_3 = \frac{1}{2\beta} - 1 \hspace{1cm} A_4 = \frac{\gamma}{\beta} - 1 \hspace{1cm} A_5 = \Delta t \left( \frac{\gamma}{2\beta} - 1 \right)$$  \hspace{2cm} (29)$$

Eq. (12) can be rearranged yielding
\[ M_{XX} \ddot{u}_X + M_{XF} \ddot{u}_F + (1 + \alpha) C_{XX} \ddot{u}_X + (1 + \alpha) C_{XF} \ddot{u}_F + (1 + \alpha) \bar{K}_{XX} \ddot{u}_X + (1 + \alpha) \bar{K}_{XF} \ddot{u}_F = F_X \]  

being \( F_X \) defined by

\[ F_X = (1 + \alpha) P_X^c - \alpha P_X^p - \alpha L_{XX} X^p_X - M_{XP} \ddot{u}_p^c - \alpha C_{XX} \ddot{u}_X^p + \alpha C_{XP} \ddot{u}_X^p + \alpha C_{XF} \ddot{u}_F^p - (1 + \alpha) \bar{K}_{XP} \ddot{u}_p^c + \alpha \bar{K}_{XF} \ddot{u}_F^p \]

The substitution of Eq.s (18), (21), (22) and (23) into Eq. (30) yields

\[ \begin{align*}
M_{XF} & \left[ \frac{1}{\beta \Delta t^2} u_F^c - \frac{1}{\beta \Delta t} \ddot{u}_F^c - \frac{1}{\beta \Delta t} \ddot{u}_F^p - \frac{1}{2 \beta} \left( \frac{1}{2} \ddot{u}_F^p \right) \right] \\
+ M_{XX} & \left[ \frac{1}{\beta \Delta t^2} u_X^c - \frac{1}{\beta \Delta t} \ddot{u}_X^c - \frac{1}{\beta \Delta t} \ddot{u}_X^p - \frac{1}{2 \beta} \left( \frac{1}{2} \ddot{u}_X^p \right) \right] \\
+ (1 + \alpha) C_{XF} & \left[ \frac{\gamma}{\beta \Delta t} u_F^c - \frac{\gamma}{\beta \Delta t} \ddot{u}_F^c + \left( 1 - \frac{\gamma}{\beta} \right) \ddot{u}_F^p + \Delta t \left( 1 - \frac{\gamma}{2 \beta} \right) \ddot{u}_F^p \right] \\
+ (1 + \alpha) C_{XX} & \left[ \frac{\gamma}{\beta \Delta t} u_X^c - \frac{\gamma}{\beta \Delta t} \ddot{u}_X^c + \left( 1 - \frac{\gamma}{\beta} \right) \ddot{u}_X^p + \Delta t \left( 1 - \frac{\gamma}{2 \beta} \right) \ddot{u}_X^p \right] \\
+ (1 + \alpha) K_{XF} & \ddot{u}_F^p + (1 + \alpha) K_{XX} \ddot{u}_X^p - (1 + \alpha) L_{XX} X^c_X = F_X
\end{align*} \]

Rearranging this equation yields
\[
\frac{1}{\beta \Delta t^2} M_{xf} + (1 + \alpha) C_{xf} \frac{\gamma}{\beta \Delta t} + (1 + \alpha) K_{xf} \right] u^c_F
\]

\[
+ \left[ \frac{1}{\beta \Delta t^2} M_{xx} + (1 + \alpha) C_{xx} \frac{\gamma}{\beta \Delta t} + (1 + \alpha) K_{xx} \right] u^\zeta_X - (1 + \alpha) L_{xx} X^\zeta =
\]

\[
\ddot{F}_x + M_{xf} \left[ \frac{1}{\beta \Delta t^2} u^\rho_f + \frac{1}{\beta \Delta t} \dot{u}^\rho_f + \left( \frac{1}{2} \right) \ddot{u}^\rho_f \right]
\]

\[
+ M_{xx} \left[ \frac{1}{\beta \Delta t^2} u^\rho_x + \frac{1}{\beta \Delta t} \dot{u}^\rho_x + \left( \frac{1}{2} \right) \ddot{u}^\rho_x \right]
\]

\[
+ (1 + \alpha) C_{xf} \left[ \frac{\gamma}{\beta \Delta t} u^\rho_p + \left( \frac{\gamma}{\beta} - 1 \right) \dot{u}^\rho_p + \Delta t \left( \frac{\gamma}{2 \beta} - 1 \right) \ddot{u}^\rho_p \right]
\]

\[
+ (1 + \alpha) C_{xx} \left[ \frac{\gamma}{\beta \Delta t} u^\rho_x + \left( \frac{\gamma}{\beta} - 1 \right) \dot{u}^\rho_x + \Delta t \left( \frac{\gamma}{2 \beta} - 1 \right) \ddot{u}^\rho_x \right]
\]

which is equivalent to the following compact form

\[
\bar{K}_{xf} u^c_F + \bar{K}_{xx} u^c_X - (1 + \alpha) L_{xx} X^\zeta = \bar{F}_x
\]

being

\[
\bar{K}_{xf} = A_0 M_{xf} + (1 + \alpha) A_1 C_{xf} + (1 + \alpha) K_{xf}
\]

\[
\bar{K}_{xx} = A_0 M_{xx} + (1 + \alpha) A_1 C_{xx} + (1 + \alpha) K_{xx}
\]

\[
\bar{F}_x = \bar{F}_x + M_{xf} \left[ A_0 u^\rho_f + A_2 \dot{u}^\rho_f + A_3 \ddot{u}^\rho_f \right] + M_{xx} \left[ A_0 u^\rho_x + A_2 \dot{u}^\rho_x + A_3 \ddot{u}^\rho_x \right] + (1 + \alpha) C_{xf} \left[ A_1 u^\rho_p + A_4 \dot{u}^\rho_p + A_5 \ddot{u}^\rho_p \right] + (1 + \alpha) C_{xx} \left[ A_1 u^\rho_x + A_4 \dot{u}^\rho_x + A_5 \ddot{u}^\rho_x \right]
\]

The parameters \( A_0 \) to \( A_5 \) are defined by Eq. (29).

In matrix notation, Eq.s (26) and (34) become

\[
\begin{bmatrix}
\bar{K}_{FF} & \bar{K}_{FX} \\
\bar{K}_{XF} & \bar{K}_{XX}
\end{bmatrix}
\begin{bmatrix}
(1 + \alpha) d_F \\
(1 + \alpha) L_{xx} X^\zeta
\end{bmatrix}
= \begin{bmatrix}
\bar{u}^c_F \\
\bar{u}^c_X
\end{bmatrix}
\]

In all the interaction degrees of freedom, and for the current time step \((t + \Delta t)\), a compatibility equation is required. The subtraction between a displacement of the vehicle and the
corresponding displacement of the structure must be equal to the gap \( g \) (see Figures 1 and 2). This compatibility equation can be written as

\[
\vec{u}_X - \vec{u}_Y = \gamma_x^c
\]  

being

\[
\vec{u}_Y = \xi_{X'} \vec{u}_{X'} + \xi_{XP} \vec{u}_P + f_{YY} Y^c_Y
\]  

In this equation, each component \( \xi_{ij} \) of \( \xi_{X'} \) corresponds to the displacement in the \( Y \) type degree of freedom \( i \) for a single unit displacement in the \( F \) type degree of freedom \( j \) (see Figure 2). The components of matrix \( \xi_{YP} \) have a similar meaning. Each component \( f_{ij} \) of \( f_{YY} \) corresponds to the displacement in the \( Y \) type degree of freedom \( i \) for a single load consisting of a unitary force on the \( Y \) type degree of freedom \( j \) (see Figure 2). All the components of the matrix \( f_{YY} \) are calculated assuming null generalized displacements in the \( F \) type and \( P \) type degrees of freedom. When the finite elements are based on the beam theory the \( f_{YY} \) matrix is not null. In finite elements whose formulation is based on shape functions the \( f_{YY} \) matrix is null.

Since the number of \( Y \) type degrees of freedom coincides with the number of \( X \) type degrees of freedom, the subscript \( Y \) may be replaced with \( X \). Substituting Eq. (6) into Eq. (39), yields

\[
\vec{u}_Y = \xi_{X'} \vec{u}_{X'} + \xi_{XP} \vec{u}_P - f_{XX} X^c_x
\]  

Substituting Eq. (40) into Eq. (38) gives

\[
-(\xi_{X'} \vec{u}_{X'} + \xi_{XP} \vec{u}_P + f_{XX} X^c_x) = \gamma_x^c + \xi_{XP} \vec{u}_P
\]  

Multiplying both members of Eq. (41) by the constant \(- (1 + \alpha)\) results in

\[
(1 + \alpha) \xi_{X'} \vec{u}_{X'} - (1 + \alpha) \xi_{XP} \vec{u}_P = -(1 + \alpha) \gamma_x^c - (1 + \alpha) \xi_{XP} \vec{u}_P
\]  

Rewriting Eqs. (37) and (42) in matrix form leads to

\[
\begin{bmatrix}
\bar{K}_{FF} & \bar{K}_{FX} & (1 + \alpha) \bar{d}_{FX} \\
\bar{K}_{XF} & \bar{K}_{XX} & -(1 + \alpha) \bar{l}_{XX} \\
(1 + \alpha) \xi_{XF} & -(1 + \alpha) \bar{l}_{XX} & -(1 + \alpha) f_{XX}
\end{bmatrix}
\begin{bmatrix}
\bar{u}_F^c \\
\bar{u}_X^c \\
X^c_x
\end{bmatrix}
= 
\begin{bmatrix}
\bar{F}_F \\
\bar{F}_X \\
\bar{g}_X
\end{bmatrix}
\]  

being

\[
\bar{g}_X = -(1 + \alpha) \gamma_x^c - (1 + \alpha) \xi_{XP} \vec{u}_P
\]
It is possible to demonstrate that in Eq. (43) the coefficient matrix of the system of linear equations is symmetric.

4. NUMERICAL EXAMPLE

In order to validate the proposed formulation, a simply supported beam subjected to a moving sprung mass is analyzed. This example is solved by the present direct method and by an iterative method proposed in [4] and [5]. The obtained results are also compared with those published in [6] and [7]. Figure 3 shows a simply supported beam subjected to a moving sprung mass. The properties of the simply supported beam coincide with those used by Yang and Wu [8], being the span $L = 25.0$ m and its geometrical and mechanical properties the following: Young’s modulus $E = 2.87$ GPa, Poisson’s ratio $\nu = 0.2$, moment of inertia $I = 2.90$ m$^4$ and mass per unit length $m = 2303$ kg/m. The beam is divided in 50 finite elements.

The sprung mass is modeled with two vertical beams. The upper beam simulates the mass $M_s = 5750$ kg and the lower beam simulates the stiffness $k = 1595 \times 10^3$ N/m of the vehicle. The constant velocity of the sprung mass is $v = 100$ km/h, its frequency is $f_v = 2.65$ Hz and its mass ratio $M_s/(mL)$ is 0.1. The damping effects of the bridge and vehicle are considered to be negligible.

In the numerical integration by the HHT method the following parameters are considered: $\Delta t = 0.005$ s, $\beta = 0.25$, $\gamma = 0.50$ and $\alpha = 0$. The total number of time steps is 180.

![Figure 3. Simply supported beam subjected to a moving sprung mass.](image)

The dynamic response of the beam subjected to the moving sprung mass, in terms of displacement and acceleration at the midpoint, is plotted in Figure 4.
The sprung mass deflection and acceleration are plotted in Figure 5.

Figure 5. Vertical deflection and acceleration of the sprung mass.

Figure 6 shows the variation of the contact force between the sprung mass and the simply supported beam.

Figure 6. Contact force between the sprung mass and the simply supported beam.

A perfect agreement can be observed between the results obtained with the proposed formulation and those published in [6] and [7].
5. CONCLUSIONS

In the present work an integrated model whose aim is the dynamic analysis of structures by the Hilber-Hughes-Taylor method is proposed. This algorithm treats the interaction between moving vehicles and a structure such as a bridge. This work provides a significant improvement relatively to the method proposed in [4] and [5], since the compatibility between vehicle and structure is no longer imposed by an iterative method, but by means of an integrated formulation that considers as variables displacements and contact forces. The governing system of equations comprises dynamic equilibrium equations and compatibility equations. The system of linear equations that arises at each time step is efficiently solved by Gaussian elimination, by considering several submatrices and their own characteristics. A significant improvement in terms of efficiency and precision could be observed.

ACKNOWLEDGMENTS

The authors wish to acknowledge the support provided by RAVE in the context of the protocol between RAVE and FEUP. This paper reports research developed under financial support provided by "FCT - Fundação para a Ciência e Tecnologia", Portugal.

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